TOWN OF HIGH LEVEL
DEVELOPMENT PERMIT

PERMIT NO.:	**DP25-030**
PROPOSED USE:	**Discretionary Use – 24,000 ft2 Riding Arena (Recreational Service – Indoor)**
APPLICANT:	**Brianne Hinson**
LANDOWNER:	**High Level Agricultural Society**
LOCATION:	**Pt. SW 3-110-19-W5**

A development involving Application No. DP25-030 has been Approved with the following Conditions.

1. **The site shall be developed in accordance with the site drawings and information attached hereto as Schedule A.**

2. **Prior to occupancy of the development, the Registered Owner/Applicant shall obtain a post-construction Lot Grading Certificate, prepared by a registered Alberta Land Surveyor, and provide the Certificate to the Development Officer. The Lot Grading Certificate must demonstrate that the post-construction lot grades, drainage, and elevations are consistent with the approved Lot Grading and Drainage Plan as per Schedule B.**

3. **Development must be commenced within one (1) year from the Date of Issue. If at the expiry of this period, the development has not commenced, this Permit shall be null and void.**

4. **The Applicant/Registered Owner shall ensure there is no damage to municipal property resulting from this permit. Costs for repairs of municipal property will be assessed by the Town of High Level and will be charged back to the applicant.**

You are hereby authorized to proceed with the development specified, provided that any stated conditions are complied with, that all other applicable permits are obtained, and that the appropriate appeal period has been exhausted. Should an appeal be made against this decision to the Subdivision and Development Appeal Board, this Development Permit shall not come into effect until the appeal has been determined and the Permit upheld, modified or nullified.

DATE OF DECISION OF DEVELOPMENT PERMIT: **September 10, 2025**

DATE OF ISSUE OF DEVELOPMENT PERMIT: **September 10, 2025**

DATE OF VALIDITY OF DEVELOPMENT PERMIT: **October 2, 2025**

SIGNATURE OF DEVELOPMENT AUTHORITY: _____

Viv Thoss

NOTES:

1. If the development is found to be incorrectly placed, the applicant may be required to move or remove the development at the sole expense of the Applicant/Registered Owner. Any changes to the attached plans will require a new development permit.

2. An appeal can be made by filing a written notice of appeal along with payment to the **Subdivision and Development Appeal Board (10511 103rd Street, High Level, AB, T0H 1Z0)** within 21 days from the date of the receipt of this decision. In the case of an appeal made by a person referred to in section 685(2) of the *Municipal Government Act*, within 21 days after the date on which the notice of the issuance of the permit was given.

3. **This is a Development Permit ONLY.** Issuance of this Permit does not excuse the applicant from satisfying all other applicable municipal, provincial and/or federal requirements.

OTHER PERMITS ARE REQUIRED

In the interest of public safety and as required by the Safety Codes Act construction permits must be obtained before commencing any work. Required permits may include building, electrical, gas, plumbing, and private sewage. Additionally, the Town of High Level requires permits for water & sewer connection, new accesses, and driveways.

PLEASE NOTE

The Applicant and/or Registered Owner are responsible for applying for, and receiving, all necessary permits prior to beginning construction. Ensure that you or your contractors obtain all other required permits related to the development. For more information regarding how to obtain the required permits, contact Superior Safety Codes 1-866-999-4777. If you are unsure which additional municipal permits you may need, please contact development@highlevel.ca.

SCHEDULE A

Approved September 10, 2025

(18 pages)

Viv Thoss
Development Authority

Development Permit Application

General Development (Non-Residential)

TOWN OF HIGH LEVEL
development@highlevel.ca
780-926-2201

Employees & Customers:

Total Staff Employed Including Business Owner	12 Volunteers	Maximum Number of Staff Present at any one Time	12 Volunteers
Expected Daily Customers	5 visitors	Expected Weekly Customers	25 visitors

Hours and Days of Operations: (Include if your operations will be seasonal)

Sunday - Sat 7:00am - 11:00pm
membership based.

Describe any storage structures and the nature of goods to be stored:

N/A

Will commercial vehicles be stored on site? How many and where?

No

What is your waste management plan?

A dumpster that a waste disposal company will provide and empty as needed.

How will local traffic be changed by this development?

Traffic will be minimally affected

Is the site open to the public? If so, what parts? (include dimensions)

Yes, community facility

Describe planned signage:

None at this time

SITE PLAN SHOWING PROPOSED DEVELOPMENT FOR

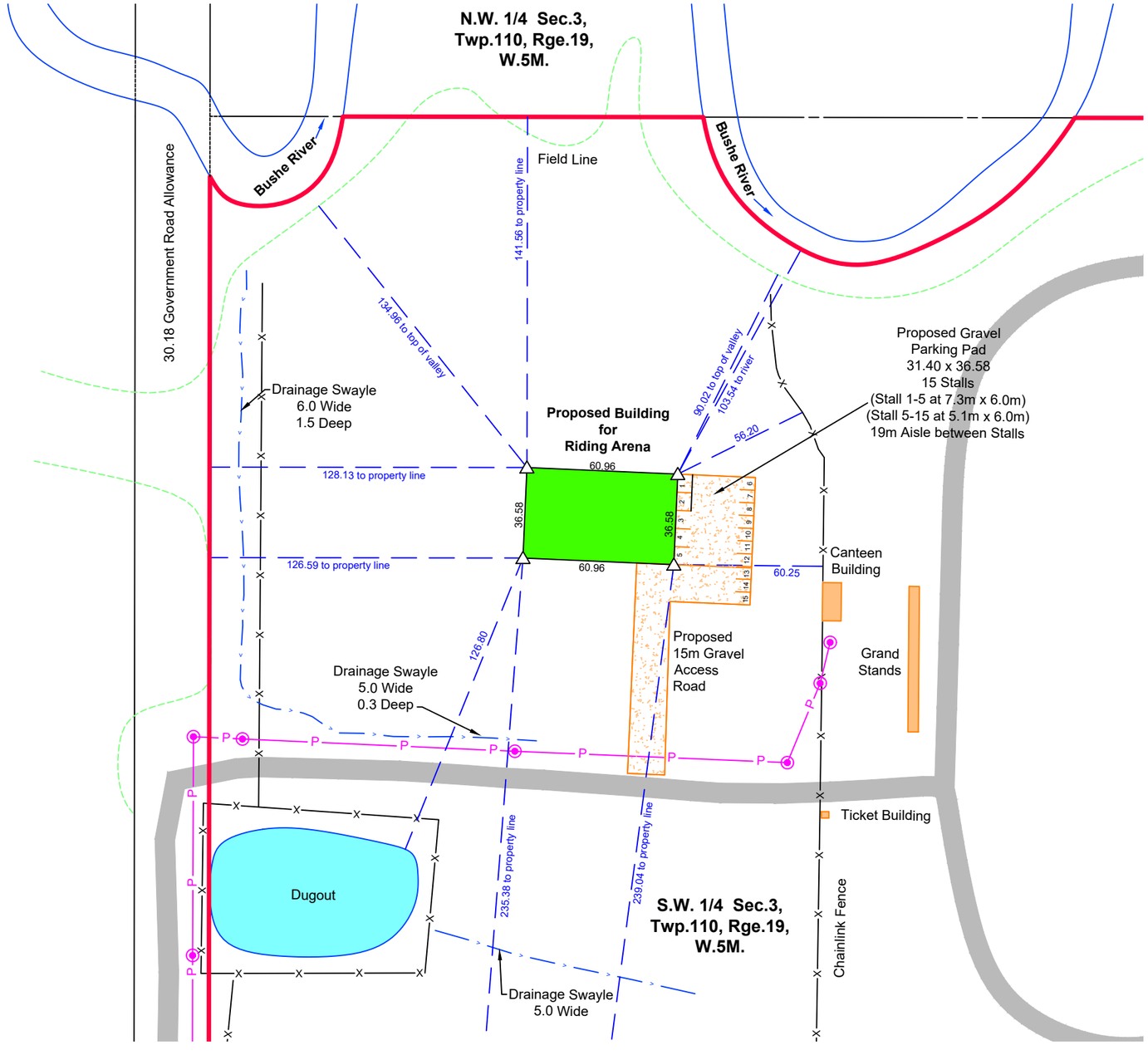
EQUESTRIAN RIDING ARENA

Within

S.W. $\frac{1}{4}$ Sec.3, Twp.110, Rge.19, W.5M.

Within

Town of High Level, Alberta



SCALE 1:2000

Notes

- Proposed Water source is a dugout.
- Proposed Septic System is a holding tank.

BORDERLINE SURVEYS

10202 99th Street
La Crete, Alberta, T0H 2H0
Phone: (780) 538-1955
E-mail: jwc.surveyor@gmail.com

ALBERTA LAND SURVEYORS' ASSOCIATION
PERMIT NUMBER P298
BORDERLINE SURVEYS LTD.

PREPARED BY

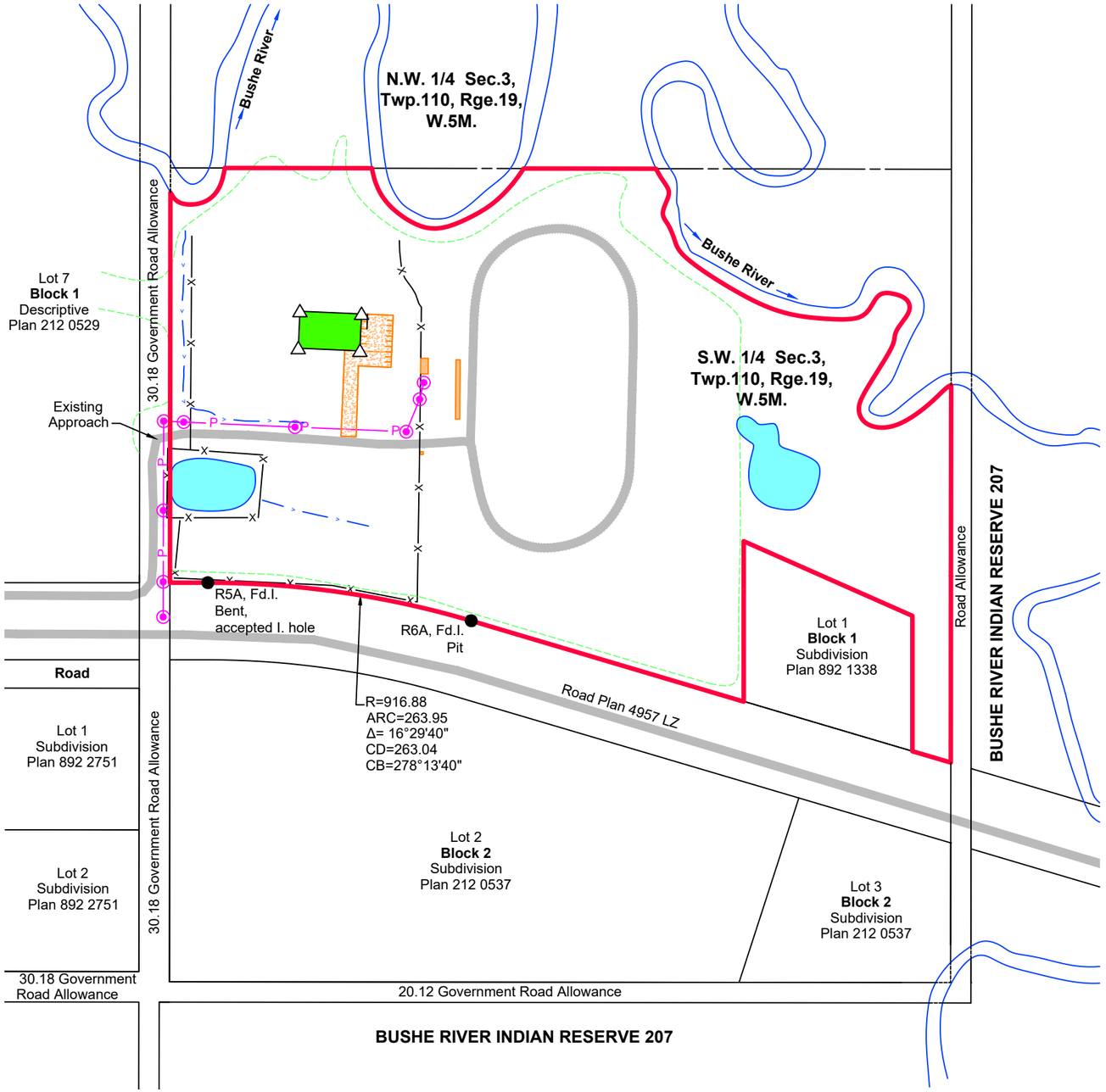
Jason Coates, A.L.S.

Revision Table

No.	Revision Type	Drafted	Chk'd	Surveyed	Date
0	Original	ASB	LB/JC	JC	June 20, 2025
1	Included Stall Aisle Width	ASB	LB/JC	JC	June 25, 2025

Client File No: N/A

File No: 250114 | Job No: 250114 | Sheet: 2 of 2 | 1 Revision



0 50 100 200 300 400 500 m

SCALE 1:5000

N

SITE PLAN SHOWING PROPOSED DEVELOPMENT FOR

EQUESTRIAN RIDING ARENA

Within

S.W. $\frac{1}{4}$ Sec.3, Twp.110, Rge.19, W.5M.

Within

Town of High Level, Alberta

Registered Title Encumbrances (Affecting Extent of Title)

912 300 865: Caveat - Easement - Alberta Power Ltd.
992 114 985: Caveat - Right of Way Agreement - ATCO Electric Ltd.

Notes

- Distances are in Metres and Decimals Thereof.
- Plan measurements based from a field inspection conducted on June 16, 2025.
- Bushe River banks and field line are digitized from Microsoft Bing Image dated October, 2024.

Legend

Area Affected by This Plan is Outlined Thus.....
Roads Shown Thus.....
Overhead Power Shown Thus.... P
Fence Shown Thus..... X
Gate Post Shown Thus..... GP
Power Pole & Anchor Shown Thus.....
Water Well/Cistern Shown Thus..... W
Septic Holding Tank Shown Thus..... S
Found Iron Post Shown Thus.....
Placed Spike Shown Thus.....

Land Owner(s)

High Level Agricultural Exhibition Association Agricultural Society
C. of T. 192 199 262

Site Information

Address: 8720 88 Street

BORDERLINE SURVEYS
10202 99th Street
La Crete, Alberta, T0H 2H0
Phone: (780) 538-1955
E-mail: jwc.surveyor@gmail.com

ALBERTA LAND SURVEYORS' ASSOCIATION
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BORDERLINE SURVEYS LTD.

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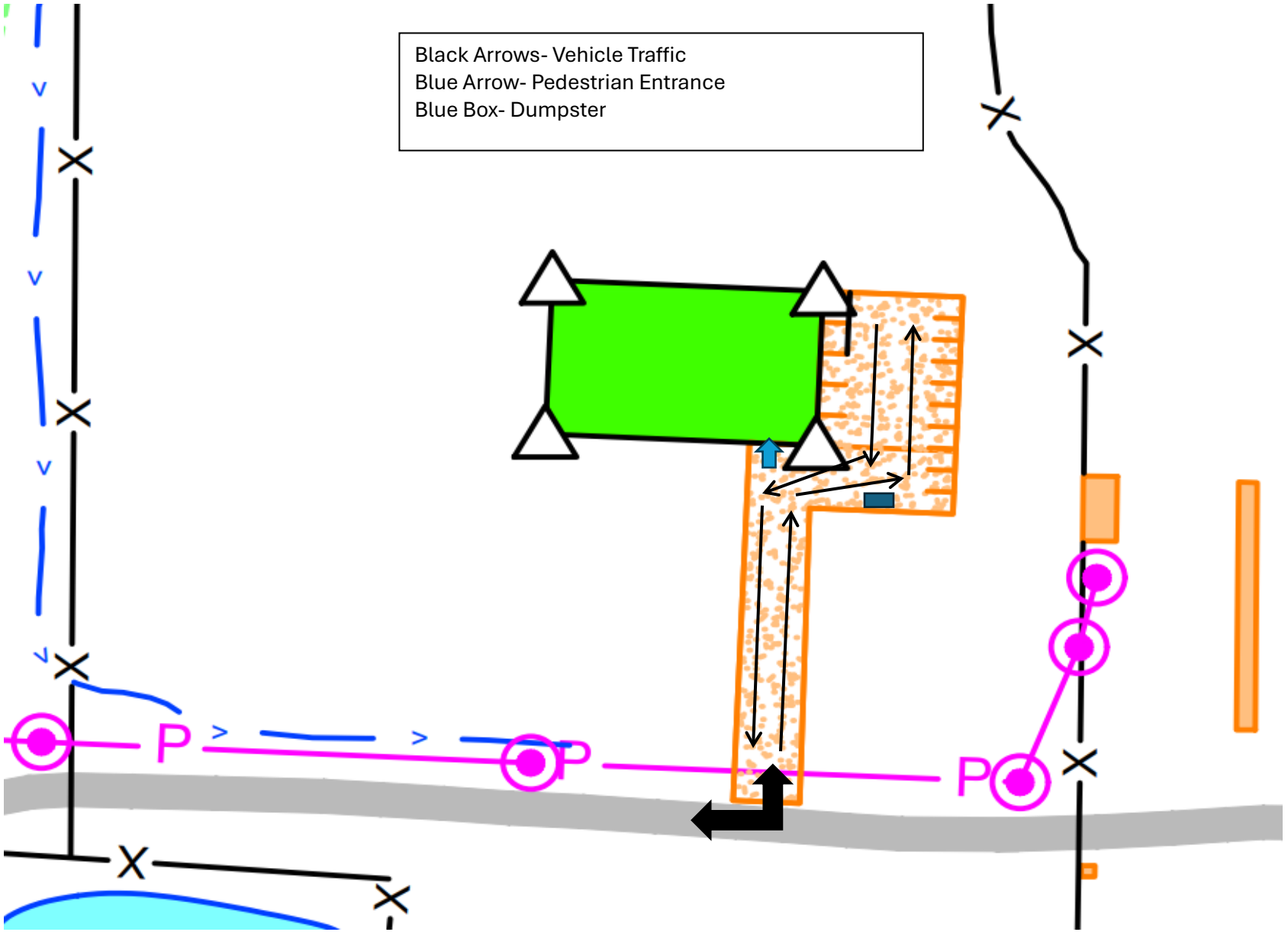
Client File No: N/A

File No: 250114 | Job No: 250114 | Sheet: 1 of 2

1 Revision

Parking & Circulation Plan



BUILDING LOADS / DESCRIPTION:

WIDTH: 120'–0" LENGTH: 200'–0" EAVE HEIGHT: 20'–0"/20'–0"
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : ABBC 2023 .

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL GOVERNING AUTHORITIES.

ROOF SLOPE:	2.0:12	
ROOF DEAD LOAD:	3.00	PSF (ROOF PANELS & PURLINS)
COLLATERAL LOAD:	2.00	PSF
ROOF LIVE LOAD:	20.89	PSF
ROOF SNOW:	40.55	PSF

SNOW Ss: 48.07 PSF RAIN Sr: 2.09 PSF

WIND LOAD 1/50: 6.27 PSF

SEISMIC DATA:

Sa(0.2):	0.0590	Sa(2.0):	0.0110	CALCULATED Fa:	1.600
Sa(0.5):	0.0380	Sa(5.0):	0.0031	CALCULATED Fv:	1.600
Sa(1.0):	0.0220	Sa(10.0):	0.0013	SITE CLASSIFICATION FOR SEISMIC SITE RESPONSE:	D

RIGID FRAME Rd:	1.5
RIGID FRAME Ro:	
SIDEWALL Rd:	1.5
SIDEWALL Ro:	

DEFLECTION LIMITS:

GIRTS:	L/120	RIGID FRAME HORIZONTAL:	H/60
PURLINS (LIVE):	L/180	RIGID FRAME VERTICAL:	L/180
PURLINS (WIND):	L/120	RIGID FRAME CRANE:	H/100
ENDWALL COLUMNS:	L/120	PARTITION COLUMN:	N/A
ENDWALL RAFTERS (LIVE):	L/180	PARTITION GIRT:	N/A
ENDWALL RAFTERS (WIND):	L/180		

MATERIAL YIELDS (MIN.):

HOT ROLLED BEAMS:	50.00	ksi
COLD ROLLED SHAPES:	50.00	ksi
BUILT–UP WEB PLATE:	55.00	ksi
BUILT–UP FLANGE BARS:	55.00	ksi
BRACING RODS & ANGLES:	44.00	ksi
HOT ROLLED CHANNELS:	44.00	ksi
HSS TUBES:	50.00	ksi

OTHER LOADS

SPECIAL NOTES:

* LOW BUILDING IMPORTANCE*

GENERAL NOTES

MATERIAL SPECIFICATIONS:

HOT ROLLED STRUCTURAL SECTIONS
BEAMS: CSA G40.21 or ASTM A992 or ASTM A572
CHANNELS: CSA G40.21
HOLLOW STRUCTURAL SECTIONS (HSS) TUBES: CSA G40.21 Class C
BUILT–UP SECTIONS
FLANGE BARS: ASTM A529 or ASTM A572
WEBS/STRUCTURAL STEEL PLATE: CSA G40.21 or ASTM A572
BRACING RODS & ANGLES: CSA G40.21
COLD–FORMED PURLINS, GIRTS, EAVE STRUTS & 'C' SECTIONS: ASTM A653
BRYTEX BUILDING SYSTEMS INC. RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIALS.

STRUCTURAL FASTENERS:

PRIMARY STEEL – MIN. 5/8" (ASTM F3125 Grade A325) BLACK
SECONDARY STEEL – MIN. 1/2" (A307) PLATED

WELDING:

CSA W59–13

PAINT:

ALL STRUCTURAL STEEL SHALL RECEIVE ONE COAT OF A RUST INHIBITIVE PRIMER (UNLESS NOTED BELOW) NOTE: RUST INHIBITIVE PRIMER IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

PREP:	SP2
PRIMER:	ONE COAT OF T101 SUN GREY PRIMER
FINISH COAT:	NONE

BOLT INSTALLATION REQUIREMENTS FOR PRIMARY STEEL CONNECTIONS AND ANCHOR RODS (BOLTS):

- ALL PARTS TO BE SOLIDLY FIT TOGETHER, WITH BOLT HOLES PROPERLY ALIGNED
- BOLTS SHALL THEN BE INSTALLED TO A SNUG–TIGHT CONDITION (THIS IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL PLIES IN A JOINT ARE IN FIRM CONTACT, WHICH IS ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A PERSON USING A SPUD WRENCH.)
- BOLTS SHALL THEN BE TIGHTENED BY THE "TURN OF THE NUT" METHOD – 1/3 TURN. (BOLTS EXCEEDING 1/3 TURN SHALL NOT BE CAUSE FOR REJECTION.)

REFER TO THE FOLLOWING SPECIFICATIONS FOR INFORMATION REGARDING ERECTION/ASSEMBLY REQUIREMENTS:

- CSA S16–14: DESIGN OF STEEL STRUCTURES
- CSA W59–13: WELDED STEEL CONSTRUCTION
- BRYTEX ERECTION MANUAL

CWB CERTIFIED CSA A660

CWB CERTIFIED CSA W47.1

APPROVAL OF THE BRYTEX BUILDING SYSTEMS INC. (BRYTEX) DRAWINGS INDICATE THAT BRYTEX CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. WHERE DISCREPANCIES EXIST BETWEEN THE BRYTEX DRAWINGS AND THE DRAWINGS FOR OTHER TRADES, THE BRYTEX DRAWINGS SHALL GOVERN. DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY BRYTEX ARE THE RESPONSIBILITY OF THE CONTRACTOR AND ENGINEERS OTHER THAN BRYTEX (UNLESS SPECIFICALLY INDICATED).

SHIPPING INFORMATION:

SITE ADDRESS:
High Level, AB
SITE CONTACT: Brianne Hinson
PHONE No. : 780–841–9195

ERECTION DRAWING SCHEDULE

SHEET No.	DESCRIPTION
CVR1	DRAWING COVER PAGE
S1	Brytex Standards
S2	Anchor Bolt Plan
S3	Anchor Bolt Details
S4	Building Reactions
S5	Rigid Frame @ Baylines 2 & 8
S6	Rigid Frame @ Baylines 3 through 7
S7	Endwall Framing @ Bayline 1
S8	Endwall Framing @ Bayline 9
S9	Sidewall Framing @ Gridline F
S10	Sidewall Framing @ Gridline A
S11	Roof Framing Plan
S12	Structural Details 1
S13	Structural Details 2

PERMIT TO PRACTICE
NOTCH1 MANAGEMENT LTD.
Signature
Date 3 September 2024
PERMIT NUMBER: 14770
The Association of Professional Engineers, Geologists and Geophysicists of Alberta

PROFESSIONAL ENGINEER KEVIN LACROIX ALBERTA

No.	REVISION	DATE	BY	CHKD
0	ISSUED FOR APPROVAL	3/29/23	SG	CP
1	ISSUED FOR FABRICATION	4/11/23	SG	CS
2	ISSUED FOR FABRICATION	4/09/24	MBB	CP

No.	REVISION	DATE	BY	CHKD

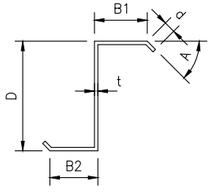
BRYTEX BUILDING SYSTEMS INC.
5610 – 97 Street
Edmonton Alberta Canada
T6E–3J1
Phone 780–437–7970
Fax 780–437–5022
www.brytex.com

CUSTOMER: NORTHERN STEEL BUILDINGS
PROJECT: HIGH LEVEL AG SOCIETY
LOCATION: HIGH LEVEL, ALBERTA
JOB No. 3378–23T

DRAWN BY:	DESIGN BY:	DWG. No.	REV. No.
SG	KL	CVR1	2

BRYTEX STANDARD COLD FORMS

CROSS SECTION



GIRTS & PURLINS

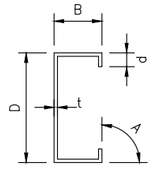
EXAMPLE: 8X25Z16

- D = DEPTH
  - 8 = 8"
  - 10 = 10"
  - 12 = 12"
  - 14 = 14"
- NOMINAL FLANGE WIDTH (25 = 2½", 35 = 3½")
- Z = Z SHAPE
- GAGE (16 = 16ga, 14 = 14ga, 12 = 12ga)

B1 = 2⅜" (25) & 3⅜" (35)
B2 = 2⅛" (25) & 3⅛" (35)
d = .945"±
A = 50°
t = .060" (16ga), .074" (14ga) & .100" (12ga)

MATERIAL THICKNESS INCLUDES STANDARD G90 COATING

GIRTS, PURLINS & FRAMED OPENINGS



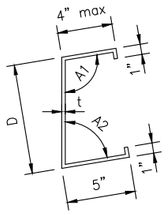
EXAMPLE: 8X25C16

- D = DEPTH
  - 8 = 8"
  - 10 = 10"
  - 12 = 12"
  - 14 = 14"
- NOMINAL FLANGE WIDTH (25 = 2½", 35 = 3½")
- C = C SHAPE
- GAGE (16 = 16ga, 14 = 14ga, 12 = 12ga)

B = 2½" (25) & 3½" (35)
d = .82"±
A = 90°
t = .060" (16ga), .074" (14ga) & .100" (12ga)

MATERIAL THICKNESS INCLUDES STANDARD G90 COATING

LOW EAVE EAVESTRUT



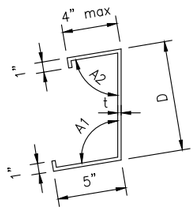
EXAMPLE: E10X20L2

- E = EAVE STRUT
- D = NOMINAL DEPTH
  - 8 = 8"
  - 10 = 10"
  - 12 = 12"
- ROOF SLOPE (20 = 2":12" etc..)
- L = LOW EAVE
- GAGE (2 = 12ga, 4 = 14ga)

A1 = 90° + ROOF SLOPE ANGLE
A2 = 90° − ROOF SLOPE ANGLE
t = .074" (14ga) & .100" (12ga)

MATERIAL THICKNESS INCLUDES STANDARD G90 COATING

HIGH EAVE EAVESTRUT



EXAMPLE: E10X20H2

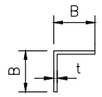
- E = EAVE STRUT
- D = NOMINAL DEPTH
  - 8 = 8"
  - 10 = 10"
  - 12 = 12"
- ROOF SLOPE (20 = 2":12" etc..)
- H = HIGH EAVE
- GAGE (2 = 12ga, 4 = 14ga)

A1 = 90° + ROOF SLOPE ANGLE
A2 = 90° − ROOF SLOPE ANGLE
t = .074" (14ga) & .100" (12ga)

MATERIAL THICKNESS INCLUDES STANDARD G90 COATING

BRYTEX STANDARD COLD FORMS

CROSS SECTION



FLANGE BRACES & PURLIN/GIRT SAG ANGLES

EXAMPLE: L225X12

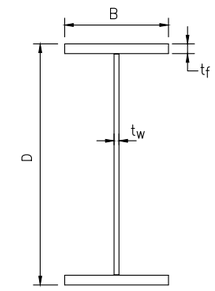
- L = FLANGE BRACE
- LEG LENGTH (225 = 2¼", 300 = 3")
- GAGE (12 = 12ga, 10 = 10ga,)

B = 2¼" (225) & 3" (300)
t = .100" (12ga) & .128" (10ga)

MATERIAL THICKNESS INCLUDES STANDARD G90 COATING

BRYTEX STANDARD 3–PLATE SHAPES (CONSTANT DEPTH BEAMS)

CROSS SECTION



PORTAL FRAMES, JACK BEAMS & FLOOR BEAMS

EXAMPLE: B1810126

- D = DEPTH (INCHES)
- B = FLANGE WIDTH (INCHES)
- t_f = FLANGE THK. (1/16" INCREMENTS) (ei. – 12 = 3/4" THK)
- t_w = WEB THK. (1/16" INCREMENTS) (ei. – 6 = 3/8" THK)

PERMIT TO PRACTICE
NOTCH1 MANAGEMENT LTD.
Signature
Date 3 September 2024
PERMIT NUMBER: 14770
The Association of Professional Engineers, Geologists and Geophysicists of Alberta

PROFESSIONAL ENGINEER ALBERTA KEVIN LACROIX

No.	REVISION	DATE	BY	CHKD
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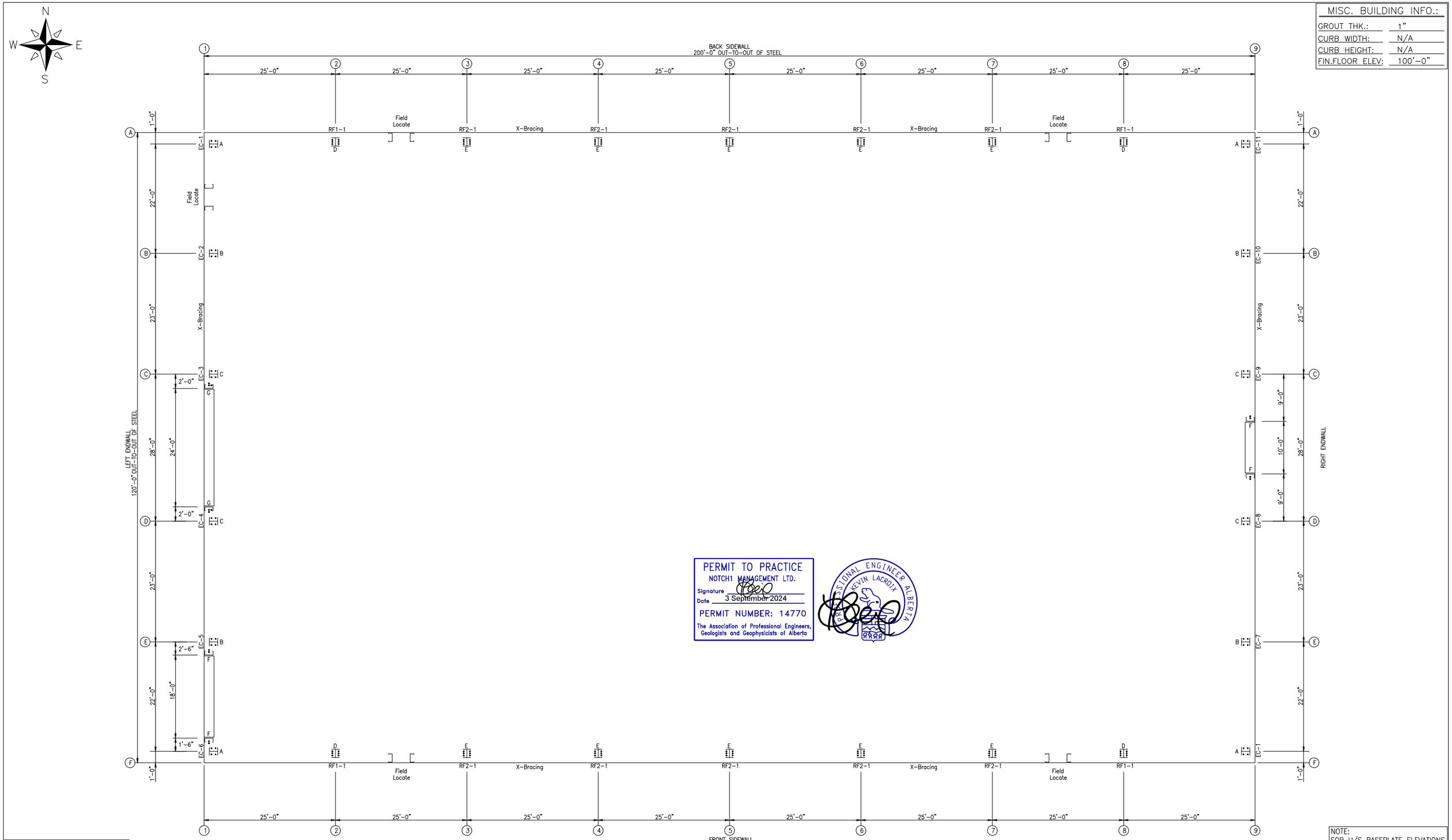
1	ISSUED FOR FABRICATION	4/11/23	SG	CS
2	ISSUED FOR FABRICATION	3/09/24	MBB	CP

No.	REVISION	DATE	BY	CHKD

BRYTEX™
BRYTEX BUILDING SYSTEMS INC.

CUSTOMER: NORTHERN STEEL BUILDINGS	DESIGN BY:	DRAWN BY:	CHKD BY:	DATE:
PROJECT: HIGH LEVEL AG SOCIETY	KL	SG	CP	3/28/23
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.:	SHEET No.:		REV.
	3378–23T	S1 OF S13		2

MISC. BUILDING INFO.:	
GROUT THK.:	1"
CURB WIDTH:	N/A
CURB HEIGHT:	N/A
FIN.FLOOR ELEV:	100'-0"



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NOTCH1 MANAGEMENT LTD.
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PROFESSIONAL ENGINEER ALBERTA
KEVIN LACROIX

ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

NOTE:
FOR U/S BASEPLATE ELEVATIONS
SEE BASEPLATE DETAIL DRAWING.

NOTE: DO NOT SCALE DRAWINGS.

No.	REVISION	DATE	BY	CHKD
0	ISSUED FOR APPROVAL	3/29/23	SG	CP
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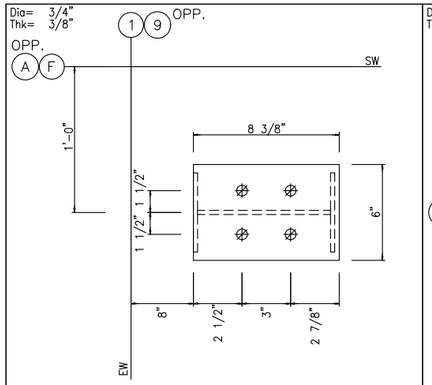
No.	REVISION	DATE	BY	CHKD

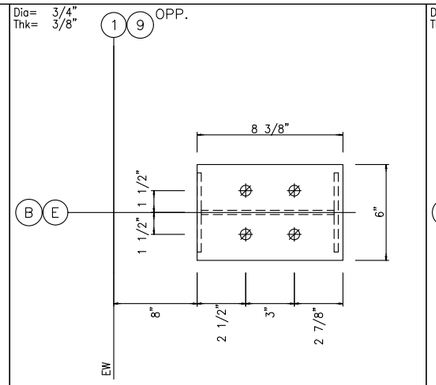
ANSI D (22"x34")

BRYTEX™
BRYTEX BUILDING SYSTEMS INC.

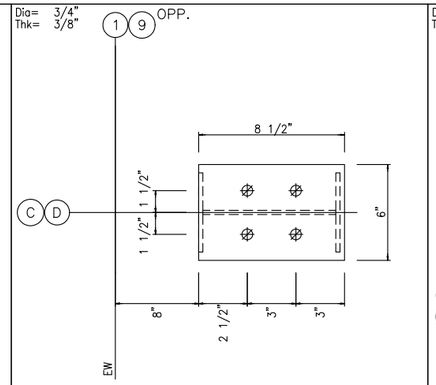
CUSTOMER: NORTHERN STEEL BUILDINGS	DESIGN BY:	DRAWN BY:	CHKD BY:	DATE:
PROJECT: HIGH LEVEL AG SOCIETY	KL	SG	CP	3/28/23
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.:	SHEET No.:		REV.
	3378-23T	S2 OF S13		2



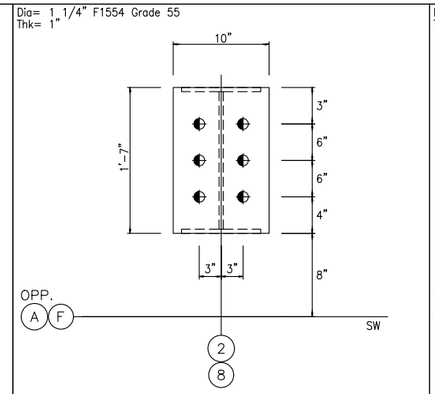
DETAIL A Base EL. 100'-1"



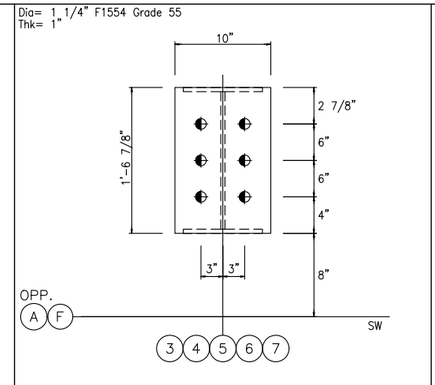
DETAIL B Base EL. 100'-1"



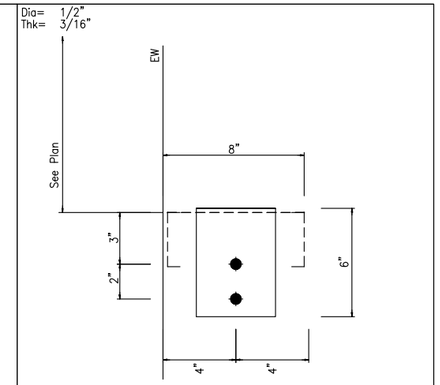
DETAIL C Base EL. 100'-1"



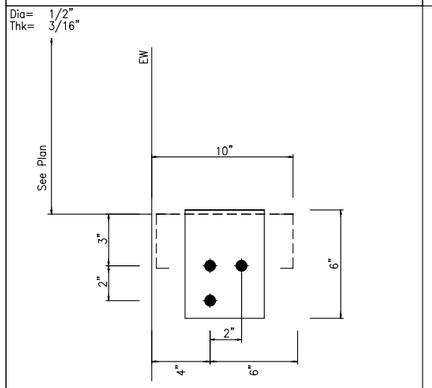
DETAIL D Base EL. 100'-1"



DETAIL E Base EL. 100'-1"



DETAIL F



DETAIL G

NOTE: DO NOT SCALE DRAWINGS.

No.	REVISION	DATE	BY	CHKD	No.	REVISION	DATE	BY	CHKD
0	ISSUED FOR APPROVAL	3/29/23	SG	CP					
1	ISSUED FOR FABRICATION	4/11/23	SG	CS					
2	ISSUED FOR FABRICATION	3/09/24	MBB	CP					

ANSI D (22"x34")

PERMIT TO PRACTICE
NOTCH1 MANAGEMENT LTD.
Signature
Date 3 September 2024
PERMIT NUMBER: 14770
The Association of Professional Engineers, Geologists and Geophysicists of Alberta

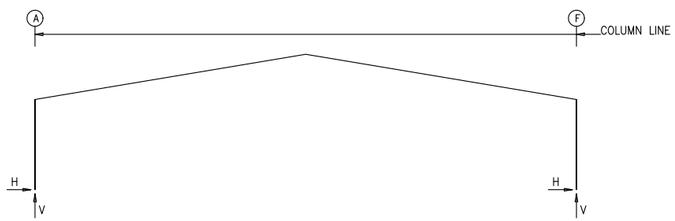
PROFESSIONAL ENGINEER KEVIN LACROIX ALBERTA

LEGEND:
"Dia=" = Anchor Rod Diameter
"Thk=" = Base Plate Thickness
"SW" = Building Sidewall
"EW" = Building Endwall
"Base EL." = U/S Of Baseplate

BRYTEX™
BRYTEX BUILDING SYSTEMS INC.

CUSTOMER: NORTHERN STEEL BUILDINGS	DESIGN BY:	DRAWN BY:	CHKD BY:	DATE:
PROJECT: HIGH LEVEL AG SOCIETY	KL	SG	CP	3/28/23
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.:	SHEET No.:		REV.
	3378-23T	S3 OF S13		2

FRAME LINES: 2 3 4 5 6 7 8



RIGID FRAME: MAXIMUM FACTORED REACTIONS

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin
2*	A	1	108.7	117.3	4	−18.2	−18.9
2*	F	5	18.2	−18.9	1	−108.7	117.3
		1	−108.7	117.3	5	18.2	−18.9

2* Frame lines: 2 8

RIGID FRAME: MAXIMUM FACTORED REACTIONS

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin
3*	A	1	99.6	106.8	4	−13.4	−13.8
		3	99.2	106.9	6	−7.7	−16.2
3*	F	5	13.4	−13.8	1	−99.6	106.8
		2	−99.2	106.9	7	7.7	−16.2

3* Frame lines: 3 4 5 6 7

RIGID FRAME: UNFACTORED COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	A	8.2	10.2	3.2	3.3	32.5	34.5	63.0	66.9	−18.2	−20.1	−13.2	−16.1
2*	F	−8.2	10.2	−3.2	3.3	−32.5	34.5	−63.0	66.9	13.2	−16.1	18.2	−20.1

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	A	−7.4	−5.4	−2.3	−1.4	−11.3	−17.0	−12.3	−14.4	−1.2	−0.3	1.2	0.3
2*	F	2.3	−1.4	7.4	−5.4	12.3	−14.4	11.3	−17.0	−1.2	0.3	1.2	−0.3

Frame Line	Column Line	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	A	47.3	58.7	47.3	41.7
2*	F	−47.3	41.7	−47.3	58.7

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
3*	A	7.6	9.4	2.9	3.0	29.7	31.3	57.7	60.8	−14.4	−15.9	−11.0	−13.1
3*	F	−7.6	9.4	−2.9	3.0	−29.7	31.3	−57.7	60.8	11.0	−13.1	14.4	−15.9

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
3*	A	−4.5	−2.6	−1.1	0.3	−10.4	−17.6	−11.2	−15.3	−1.1	−0.3	1.1	0.3
3*	F	1.1	0.3	4.5	−2.6	11.2	−15.3	10.4	−17.6	−1.1	0.3	1.1	−0.3

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	F2UNB_SL_L Horiz	F2UNB_SL_L Vert	F2UNB_SL_R Horiz	F2UNB_SL_R Vert
3*	A	0.0	−5.7	43.3	53.3	43.3	37.9
3*	F	0.0	−5.7	−43.3	37.9	−43.3	53.3

2* Frame lines: 2 8
3* Frame lines: 3 4 5 6 7

ENDWALL COLUMN: UNFACTORED COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	A	0.7	0.3	2.6	5.0	0.0	−1.8	0.0	−1.6	0.0	−0.4	0.0	−0.2	−1.1
1	B	1.5	0.6	6.7	13.0	−2.0	−7.6	0.0	−0.7	−2.0	−4.5	0.0	2.4	−2.4
1	C	1.7	0.7	7.1	13.7	0.0	−2.5	2.0	−5.7	0.0	−0.1	2.0	−3.3	−3.2
1	D	1.7	0.7	7.1	13.7	0.0	−3.0	0.0	−4.9	0.0	−0.6	0.0	−2.4	−3.2
1	E	1.5	0.6	6.7	13.0	0.0	−3.4	0.0	−5.2	0.0	−0.3	0.0	−2.1	−2.4
1	F	0.7	0.3	2.6	5.0	0.0	−1.6	0.0	−1.8	0.0	−0.2	0.0	−0.4	−1.1

Frm Line	Col Line	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	E1UNB_SL_L− Horz	E1UNB_SL_L− Vert	E1UNB_SL_R− Horz	E1UNB_SL_R− Vert
1	A	0.7	−2.2	−2.2	0.0	0.0	0.0	0.0	0.0	5.0	0.0	2.5
1	B	1.7	−5.3	−5.3	−1.2	−1.4	0.0	1.6	0.0	13.2	0.0	6.3
1	C	2.2	−4.6	−4.6	0.0	1.3	1.2	−1.5	0.0	12.9	0.0	7.7
1	D	2.2	−4.6	−4.6	0.0	0.1	0.0	−0.1	0.0	7.7	0.0	12.9
1	E	1.7	−5.3	−5.3	0.0	−0.1	0.0	0.1	0.0	6.3	0.0	13.2
1	F	0.7	−2.2	−2.2	0.0	0.0	0.0	0.0	0.0	2.5	0.0	5.0

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
9	F	0.7	0.3	2.6	5.0	0.0	−1.8	0.0	−1.6	0.0	−0.4	0.0	−0.2	−1.1
9	E	1.5	0.6	6.7	13.0	0.0	−5.2	0.0	−3.4	0.0	−2.1	0.0	−0.3	−2.4
9	D	1.7	0.7	7.1	13.7	0.0	−4.9	0.0	−3.0	0.0	−2.4	0.0	−0.6	−3.2
9	C	1.7	0.7	7.1	13.7	−2.0	−5.7	0.0	−2.5	−2.0	−3.3	0.0	−0.1	−3.2
9	B	1.5	0.6	6.7	13.0	0.0	−0.7	2.0	−7.6	0.0	2.4	2.0	−4.5	−2.4
9	A	0.7	0.3	2.6	5.0	0.0	−1.6	0.0	−1.8	0.0	−0.2	0.0	−0.4	−1.1

Frm Line	Col Line	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	E2UNB_SL_L− Horz	E2UNB_SL_L− Vert	E2UNB_SL_R− Horz	E2UNB_SL_R− Vert
9	F	0.7	−2.2	−2.2	0.0	0.0	0.0	0.0	0.0	5.0	0.0	2.5
9	E	1.7	−5.3	−5.3	0.0	0.1	0.0	−0.1	0.0	13.2	0.0	6.3
9	D	2.2	−4.6	−4.6	0.0	0.1	0.0	0.1	0.0	12.9	0.0	7.7
9	C	2.2	−4.6	−4.6	−1.2	−1.5	0.0	1.3	0.0	7.7	0.0	12.9
9	B	1.7	−5.3	−5.3	0.0	1.6	1.2	−1.4	0.0	6.3	0.0	13.2
9	A	0.7	−2.2	−2.2	0.0	0.0	0.0	0.0	0.0	2.5	0.0	5.0

ENDWALL COLUMN: MAXIMUM FACTORED REACTIONS

Frm Line	Col Line	Column_Reactions(k) Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin
1	A	8	1.0	−2.5	9	−1.5	−2.5
		1	0.0	8.7			
1	B	10	2.4	−9.2	11	−3.4	−9.2
		12	−1.0	23.1			
1	C	13	3.1	−6.5	14	−4.5	−6.5
		1	0.0	23.6			
1	D	13	3.1	−5.3	14	−4.5	−5.3
		1	0.0	23.6			
1	E	8	2.4	−6.0	9	−3.4	−6.0
		15	0.0	22.5			
1	F	8	1.0	−2.5	9	−1.5	−2.5
		1	0.0	8.7			
9	F	8	1.0	−2.5	9	−1.5	−2.5
		1	0.0	8.7			
9	E	8	2.4	−6.0	9	−3.4	−6.0
		16	0.0	22.5			
9	D	10	3.1	−5.3	11	−4.5	−5.3
		1	0.0	23.6			
9	C	10	3.1	−6.5	11	−4.5	−6.5
		1	0.0	23.6			
9	B	13	2.4	−9.2	14	−3.4	−9.2
		17	−1.0	23.1			
9	A	8	1.0	−2.5	9	−1.5	−2.5
		1	0.0	8.7			

UNFACTORED BRACING REACTIONS

Wall Loc	Wall Line	Col Line	Reactions in plane of wall ± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis
L_EW	1	B,C	Bracing, see EW reactions					
F_SW	F	3,4	3.1	*	7.9	*		
		6,7	3.1	*	7.9	*		
R_EW	9	C,B	Bracing, see EW reactions					
B_SW	A	7,6	3.1	*	7.9	*		
		4,3	3.1	*	7.9	*		

*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

GENERAL NOTES

1. Foundation design and construction are not the responsibilty of BRYTEX BUILDING SYSTEMS INC.
2. The building reaction data reports the loads which this building places on the foundation.
3. Column reactions are in kilonewtons (kN) or thousands of pounds (k), and should be considered reversible when loads are applied from the opposite side.
4. The forces noted as UNFACTORED BRACING REACTIONS are calculated as follows:
   - Wind bracing reactions are service loads.
   - Seismic bracing reactions are ultimate loads.
5. The forces noted as UNFACTORED PORTAL FRAME REACTIONS (if used) are calculated as follows:
   - Wind portal reactions are service loads.
   - Seismic portal reactions are ultimate loads.
6. The forces noted as UNFACTORED FIXED-BASE COLUMN REACTIONS (if used) are calculated as follows:
   - Wind fixed-base column reactions are service loads.
   - Seismic fixed-base column reactions are ultimate loads.
7. All crane reactions (if used) do not include any increase for impact,
8. Anchor bolts/rods shall be accurately set to a tolerance of +/− 1/8" in both elevation and location.
9. Anchor bolts/rods and embedded items are not provided by BRYTEX BUILDING SYSTEMS INC.
10. Column base plates are designed and sized to be supported on a foundation with a minimum concrete strength of f'c = 25 MPa.
11. Anchor bolts/rods are sized based on the steel strength of ASTM F1554 Grade 36 material. Anchorage of the anchor bolts/rods is the responsibiltiy of the foundation designer.
12. Load combination abbreviations:

DL	Dead Load
CL	Collateral Load
LL	Live Load
SL	Snow Load
WL	Wind Load
WP	Wind Load Pressure
WS	Wind Load Suction
WL1	Wind Load from Left plus Internal Pressure
WR1	Wind Load from Right plus Internal Pressure
WL2	Wind Load from Left plus Internal Suction
WR2	Wind Load from Right plus Internal Suction
F1UNB_SL	Frame Unbalanced Snow Load Case
LnWnd1	Longitudinal Wind Load plus Internal Pressure
LnWnd2	Longitudinal Wind Load plus Internal Suction
E1UNB_SL	End Frame Unbalanced Snow Load Case
SeisL	Seismic Load from Left
SeisR	Seismic Load from Right
LnSeis	Longitudinal Seismic
F1PAT_SL	Frame Pattern Snow Load Case

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

Width (ft)	=	120.0
Length (ft)	=	200.0
Eave Height (ft)	=	20.0/ 20.0
Roof Slope (rise/12)	=	2.0/ 2.0
Dead Load (psf)	=	3.0
Collateral Load (psf)	=	2.0
Live Load (psf)	=	20.9
Snow Load (psf)	=	40.5
Wind Load 1:50(psf)	=	6.3
Wind Load 1:30(psf)	=	6.3
Wind Code	=	NBC2023AE(NBC2020)
Wind Category		2
Exposure	=	O
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Category	=	A
Seismic Data		
Sa(0.2,X)	=	0.1300
Sa(0.5,X)	=	0.1240
Sa(1.0,X)	=	0.0696
Sa(2.0,X)	=	0.0333
Sa(5.0,X)	=	0.0073
Sa(10.0,X)	=	0.0023
PGA(X)	=	0.0695
S(2.0)	=	0.0333
S(0.5)	=	0.1240

5. Governing loading combinations are:
   1 1.25Dead+1.25Collateral+1.5Snow+1.5Snow_Drift+1.5Slide_Snow
   2 1.25Dead+1.25Collateral+1.5Snow+0.4Wind_Left2+1.5Snow_Drift+1.5Slide_Snow
   3 1.25Dead+1.25Collateral+1.5Snow+0.4Wind_Right2+1.5Snow_Drift+1.5Slide_Snow
   4 0.9Dead+1.4Wind_Left1
   5 0.9Dead+1.4Wind_Right1
   6 0.9Dead+1.4Wind_Long1L
   7 0.9Dead+1.4Wind_Long2L
   8 0.9Dead+1.4Wind_Suction+1.4Wind_Long2L
   9 0.9Dead+1.4Wind_Pressure+1.4Wind_Long2L
   10 0.9Dead+1.4Wind_Left1+1.4Wind_Suction
   11 0.9Dead+1.4Wind_Left1+1.4Wind_Pressure
   12 1.25Dead+1.25Collateral+1.5Snow+0.4Wind_Right2+0.4Wind_Pressure+1.5Snow_Drift+1.5Slide_Snow
   13 0.9Dead+1.4Wind_Right1+1.4Wind_Suction
   14 0.9Dead+1.4Wind_Right1+1.4Wind_Pressure
   15 1.25Dead+1.25Collateral+1.5E1UNB_SL_R
   16 1.25Dead+1.25Collateral+1.5E2UNB_SL_L
   17 1.25Dead+1.25Collateral+1.5Snow+0.4Wind_Left2+0.4Wind_Pressure+1.5Snow_Drift+1.5Slide_Snow

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Signature
Date 3 September 2024
PERMIT NUMBER: 14770
The Association of Professional Engineers, Geologists and Geophysicists of Alberta

PROFESSIONAL ENGINEER ALBERTA KEVIN LACROIX

No.	REVISION	DATE	BY	CHKD
0	ISSUED FOR APPROVAL	3/29/23	SG	CP
1	ISSUED FOR FABRICATION	4/11/23	SG	CS
2	ISSUED FOR FABRICATION	3/09/24	MBB	CP

ANSI D (22"x34")

BRYTEX™ BUILDING SYSTEMS INC.

CUSTOMER: NORTHERN STEEL BUILDINGS	DESIGN BY: KL	DRAWN BY: SG	CHKD BY: CP	DATE: 3/28/23
PROJECT: HIGH LEVEL AG SOCIETY				
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.: 3378−23T	SHEET No.: S4 OF S13		REV. 2

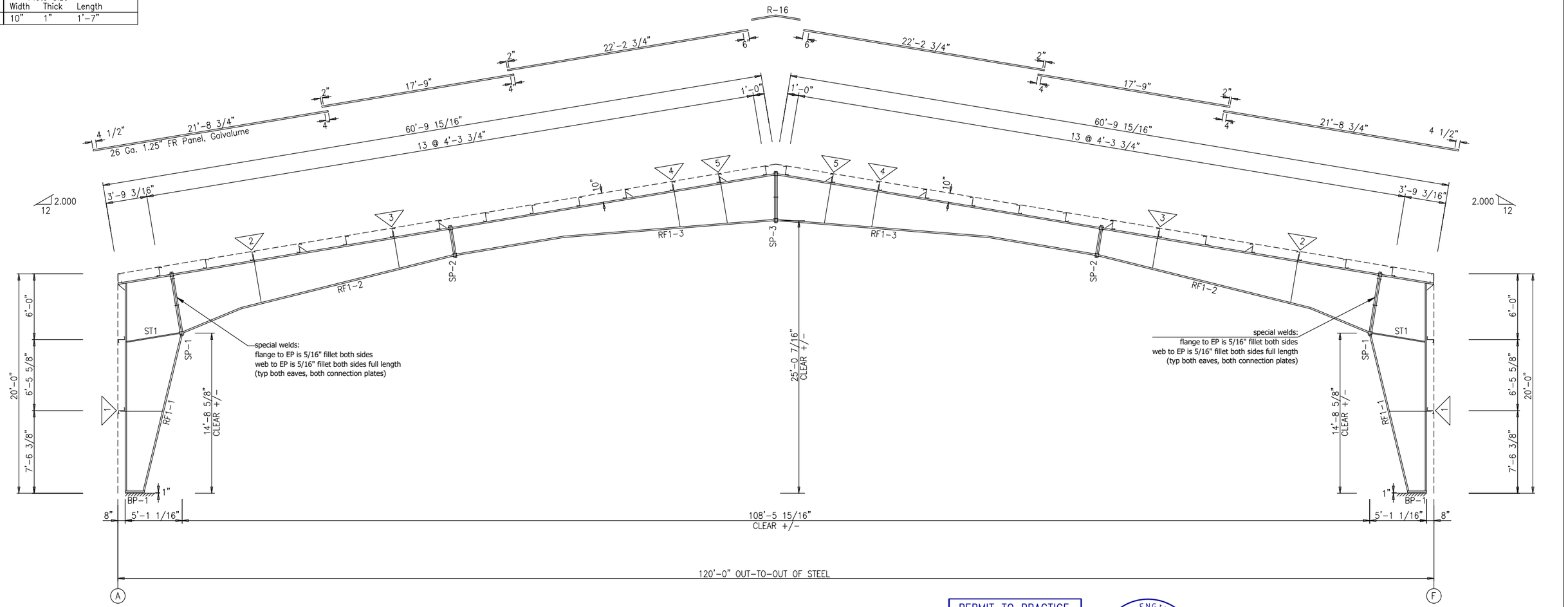
SPLICE PLATE & BOLT TABLE									
SPLICE MARK	QUANTITY OF CONNECTION BOLTS TOP	BOT	INT	TYPE	DIA	LENGTH	PLATE SIZE WIDTH	THICK	LENGTH
SP-1	6	4	2	A325	0.875	3.000	8"	3/4"	5'-11 1/16"
SP-2	4	4	0	A325	0.750	2.250	8"	1/2"	2'-11 5/8"
SP-3	4	4	2	A325	0.750	2.250	8"	1/2"	4'-8 1/8"

FLANGE BRACE TABLE				
▽ ID	# SIDES	MARK	SIZE	BRACE DISTANCE
1	2	FB6A	L225X12	2'-4"
2	2	FB11A	L225X12	3'-4"
3	2	FB4A	L225X12	2'-4"
4	2	FB7A	L225X12	2'-4"
5	2	FB10A	L225X12	3'-4"

STIFFENER TABLE				
Mark	Stiff Mark	Plate Size Width	Thick	Length
RF1-1	ST1	4.000	0.500	60.10

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	10"	1"	1'-7"

MEMBER TABLE						
Mark	Weight	Web Depth Start/End	Web Plate Thick	Length	Outside Flange Thk x W x Length	Inside Flange Thk x W x Length
RF1-1	1864	17.9/59.9	0.375	237.0	3/8" x 8 x 228.8 3/8" x 8 x 57.8	3/4" x 8 x 175.7
RF1-2	2176	62.8/47.0 47.0/28.0	0.375 0.313	68.9 240.0	3/8" x 8 x 308.9	3/4" x 8 x 70.6 3/4" x 8 x 240.6
RF1-3	1714	28.0/28.0 28.0/48.0	0.250 0.250	120.0 240.0	1/2" x 8 x 360.0	3/8" x 8 x 120.0 3/8" x 8 x 232.8



RIGID FRAME ELEVATION: FRAME LINE 2 8

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NOTCH1 MANAGEMENT LTD.
Signature
Date 3 September 2024
PERMIT NUMBER: 14770
The Association of Professional Engineers, Geologists and Geophysicists of Alberta

PROFESSIONAL ENGINEER KEVIN LACROIX ALBERTA

NOTE: DO NOT SCALE DRAWINGS.

No.	REVISION	DATE	BY	CHKD
0	ISSUED FOR APPROVAL	3/29/23	SG	CP
1	ISSUED FOR FABRICATION	4/11/23	SG	CS
2	ISSUED FOR FABRICATION	3/09/24	MBB	CP

ANSI D (22"x34")

BRYTEX™
BRYTEX BUILDING SYSTEMS INC.

CUSTOMER: NORTHERN STEEL BUILDINGS	DESIGN BY:	DRAWN BY:	CHKD BY:	DATE:
PROJECT: HIGH LEVEL AG SOCIETY	KL	SG	CP	3/28/23
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.:	SHEET No.:		REV.
	3378-23T	S5 OF S13		2

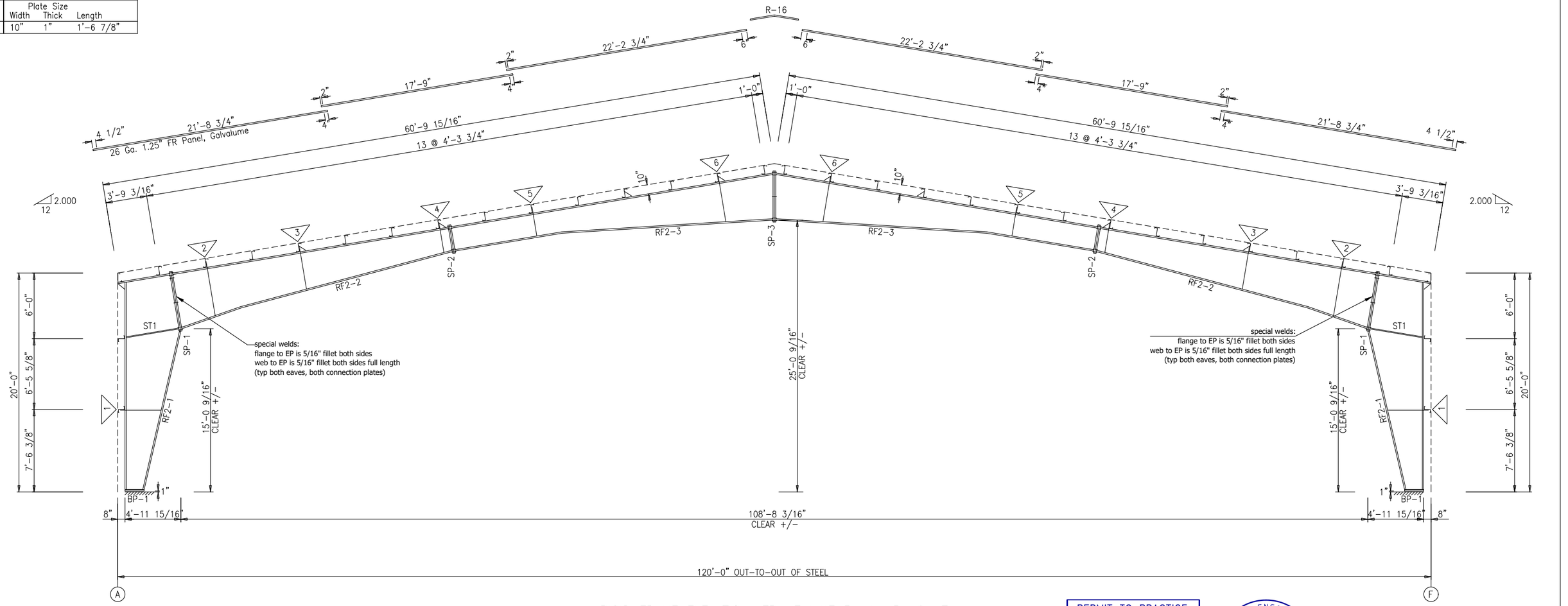
SPLICE PLATE & BOLT TABLE									
SPLICE MARK	QUANTITY OF CONNECTION BOLTS TOP	BOT	INT	TYPE	DIA	LENGTH	PLATE SIZE WIDTH	THICK	LENGTH
SP-1	6	4	2	A325	0.875	3.000	8"	3/4"	5'-7"
SP-2	4	4	0	A325	0.750	2.250	8"	1/2"	2'-7 3/4"
SP-3	4	4	2	A325	0.750	2.000	6"	3/8"	4'-8 3/16"

FLANGE BRACE TABLE				
▽ ID	# SIDES	MARK	SIZE	BRACE DISTANCE
1	2	FB5A	L225X12	2'-4"
2	2	FB12A	L225X12	3'-4"
3	2	FB8A	L225X12	2'-4"
4	2	FB3A	L225X12	2'-4"
5	1	FB2A	L225X12	2'-4"
6	2	FB9A	L225X12	3'-4"

STIFFENER TABLE					
Mark	Stiff Mark	Plate Size Width	Thick	Length	
RF2-1	ST1	4.000	0.500	59.08	

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	10"	1"	1'-6 7/8"

MEMBER TABLE						
Mark	Weight	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange Thk x W x Length	Inside Flange Thk x W x Length
RF2-1	1790	17.9/58.9	0.375	237.0	3/8" x 8 x 228.8 3/8" x 8 x 57.4	5/8" x 8 x 179.4
RF2-2	2091	58.9/47.0 47.0/24.0	0.375 0.313	69.3 240.0	3/8" x 8 x 309.3	5/8" x 8 x 70.3 3/4" x 8 x 241.0
RF2-3	1490	24.0/24.0 24.0/48.0	0.188 0.250	120.0 240.0	3/4" x 6 x 360.0	3/8" x 6 x 120.0 1/4" x 6 x 233.2



RIGID FRAME ELEVATION: FRAME LINE 3 4 5 6 7

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NOTE: DO NOT SCALE DRAWINGS.

No.	REVISION	DATE	BY	CHKD
0	ISSUED FOR APPROVAL	3/29/23	SG	CP
1	ISSUED FOR FABRICATION	4/11/23	SG	CS
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ANSI D (22"x34")

BRYTEX™
BRYTEX BUILDING SYSTEMS INC.

CUSTOMER: NORTHERN STEEL BUILDINGS	DESIGN BY:	DRAWN BY:	CHKD BY:	DATE:
PROJECT: HIGH LEVEL AG SOCIETY	KL	SG	CP	3/28/23
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.:	SHEET No.:		REV.
	3378-23T	S6 OF S13		2

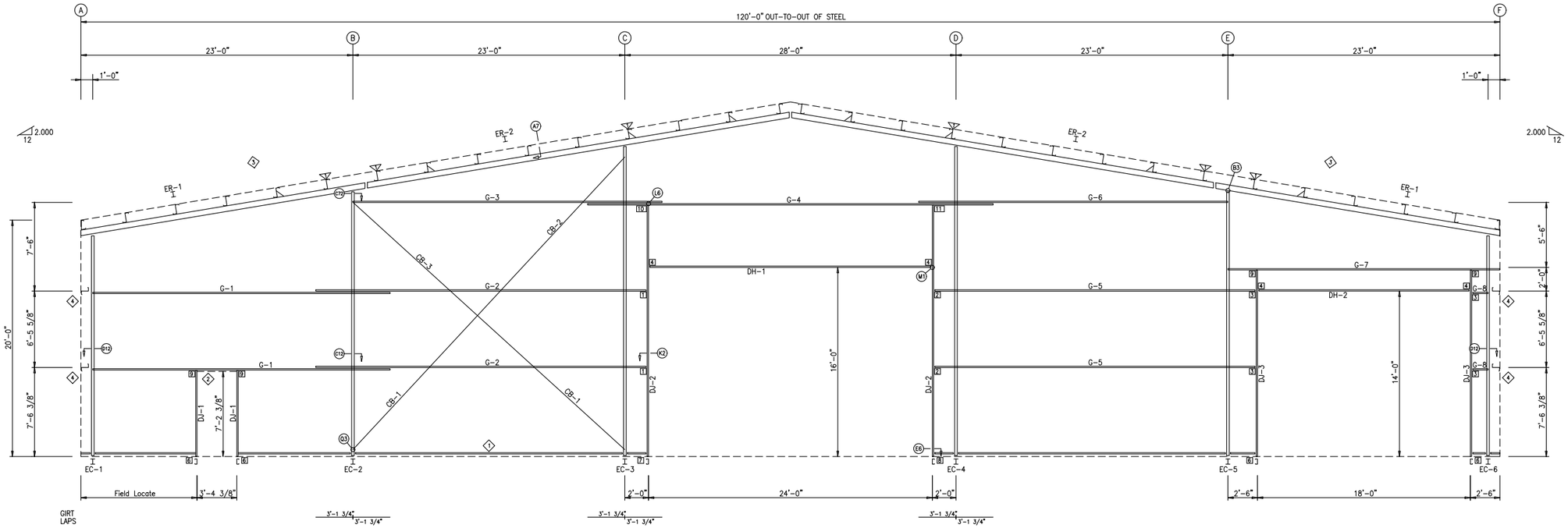
SPLICE PLATE & BOLT TABLE FRAME LINE 1							
LOCATION	CONNECTION BOLTS QUAN	TYPE	DIA	LENGTH	PLATE SIZE WIDTH	THICK	LENGTH
ER-1/ER-2	8	A325	5/8"	2"	5"	3/8"	1'-6 11/16"
ER-2/ER-2	8	A325	5/8"	2 1/4"	5"	1/2"	1'-6 7/8"
Columns/Raf	4	A325	5/8"	1 3/4"	6"	3/8"	7 7/8"
Columns/Raf	4	A325	5/8"	1 3/4"	6"	3/8"	8"

FLANGE BRACE TABLE FRAME LINE 1			
▽ ID	MARK	SIZE	BRACE DIST
1	FB1A	L225X12	1'-0"

MEMBER TABLE FRAME LINE 1		
QUAN	MARK	PART
1	EC-1	W8X10
1	EC-2	W8X10
1	EC-3	W8X13
1	EC-4	W8X13
1	EC-5	W8X10
1	EC-6	W8X10
2	ER-1	W12X14
2	ER-2	W12X14
2	DJ-1	8X25C16
2	DJ-2	10X25C12
2	DJ-3	8X35C16
1	DH-1	10X25C14
1	DH-2	8X25C16
2	G-1	8X25Z16
2	G-2	8X25Z16
1	G-3	8X25Z14
1	G-4	8X25Z16
2	G-5	8X25Z12
1	G-6	8X25Z14
1	G-7	8X25Z12
2	G-8	8X25Z16
2	CB-1	0.50_ROD
1	CB-2	0.50_ROD
1	CB-3	0.50_ROD

CONNECTION PLATES FRAME LINE 1		
□ID	QUAN	MARK/PART
1	2	GC25
2	2	GC24
3	4	GC03
4	4	GC01
6	4	GC05
7	1	GC32
8	1	GC33
9	4	GC04
10	1	GC29
11	1	GC28

ANGLE TABLE FRAME LINE 1	
◇ID	PART
1	BA-24
2	GW-8
3	RA-24
4	CA-8



LEFT ENDWALL FRAMING: FRAME LINE 1

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ANSI D (22"x34")

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PROJECT: HIGH LEVEL AG SOCIETY	KL	SG	CP	3/28/23
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.:	SHEET No.:		REV.
	3378-23T	S7 OF S13		2

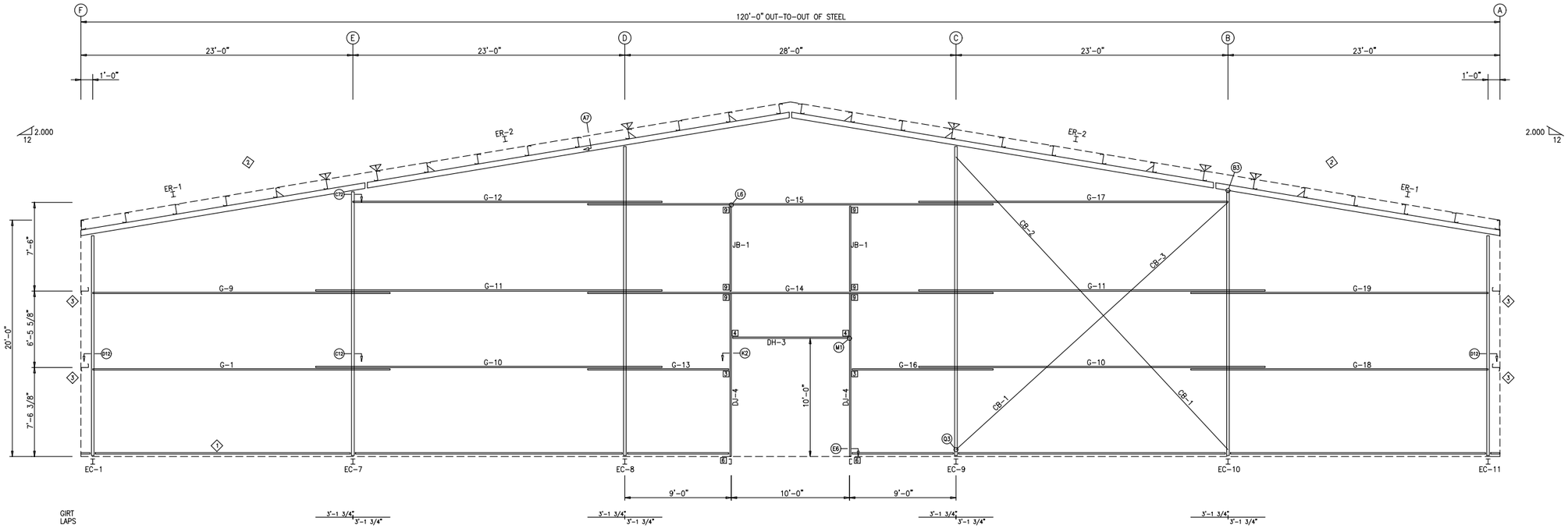
SPLICE PLATE & BOLT TABLE FRAME LINE 9							
LOCATION	CONNECTION BOLTS QUAN	TYPE	DIA	LENGTH	PLATE SIZE WIDTH	THICK	LENGTH
ER-1/ER-2	8	A325	5/8"	2"	5"	3/8"	1'-6 11/16"
ER-2/ER-2	8	A325	5/8"	2 1/4"	5"	1/2"	1'-6 7/8"
Columns/Raf	4	A325	5/8"	1 3/4"	6"	3/8"	7 7/8"
Columns/Raf	4	A325	5/8"	1 3/4"	6"	3/8"	8"

FLANGE BRACE TABLE FRAME LINE 9			
▽ ID	MARK	SIZE	BRACE DIST
1	FB1A	L225X12	1'-0"

MEMBER TABLE FRAME LINE 9		
QUAN	MARK	PART
1	EC-1	W8X10
1	EC-7	W8X10
1	EC-8	W8X13
1	EC-9	W8X13
1	EC-10	W8X10
1	EC-11	W8X10
2	ER-1	W12X14
2	ER-2	W12X14
2	DJ-4	8X25C16
1	DH-3	8X25C16
1	G-1	8X25Z16
1	G-9	8X35Z16
2	G-10	8X25Z16
2	G-11	8X35Z14
1	G-12	8X25Z16
1	G-13	8X25Z16
1	G-14	8X35Z12
1	G-15	8X25Z16
1	G-16	8X25Z16
1	G-17	8X25Z16
1	G-18	8X25Z16
1	G-19	8X35Z16
2	CB-1	0.50_ROD
1	CB-2	0.50_ROD
1	CB-3	0.50_ROD
2	JB-1	8X25C16

CONNECTION PLATES FRAME LINE 9		
□ID	QUAN	MARK/PART
3	2	GC03
4	2	GC01
6	2	GC05
9	6	GC04

ANGLE TABLE FRAME LINE 9	
◇ID	PART
1	BA-24
2	RA-24
3	CA-8



RIGHT ENDWALL FRAMING: FRAME LINE 9

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No.	REVISION	DATE	BY	CHKD

ANSI D (22"x34")

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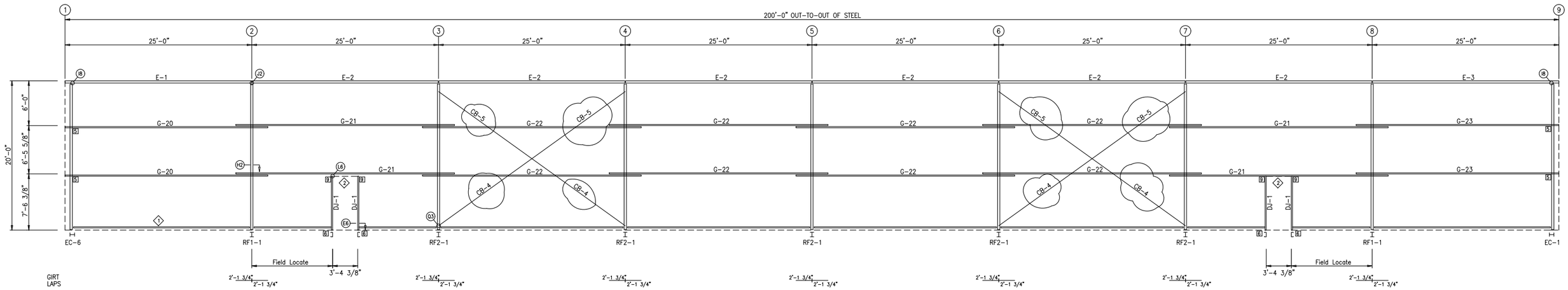
	3378-23T	S8 OF S13		2

MEMBER TABLE FRAME LINE F		
QUAN	MARK	PART
4	DJ-1	8X25C16
1	E-1	E10X20L2
6	E-2	E10X20L2
1	E-3	E10X20L2
2	G-20	8X25Z14
4	G-21	8X25Z14
8	G-22	8X25Z16
2	G-23	8X25Z14
4	CB-4	0.625_ROD
4	CB-5	0.625_ROD

CONNECTION PLATES FRAME LINE F		
□ID	QUAN	MARK/PART
5	4	GC08
6	4	GC05
9	4	GC04

ANGLE TABLE FRAME LINE F	
◇ID	PART
1	BA-24
2	GW-8



FRONT SIDEWALL FRAMING: FRAME LINE F

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PROFESSIONAL ENGINEER ALBERTA
KEVIN LACROIX

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2	ISSUED FOR FABRICATION	3/09/24	MBB	CP

No.	REVISION	DATE	BY	CHKD

ANSI D (22"x34")

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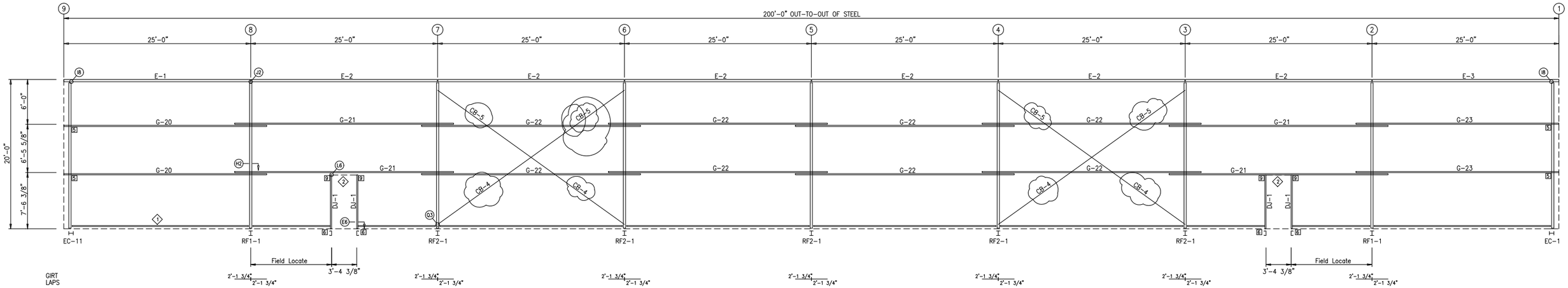
	3378-23T	S9 OF S13		2

MEMBER TABLE FRAME LINE A		
QUAN	MARK	PART
4	DJ-1	8X25C16
1	E-1	E10X20L2
6	E-2	E10X20L2
1	E-3	E10X20L2
2	G-20	8X25Z14
4	G-21	8X25Z14
8	G-22	8X25Z16
2	G-23	8X25Z14
4	CB-4	0.625_ROD
4	CB-5	0.625_ROD

CONNECTION PLATES FRAME LINE A		
□ID	QUAN	MARK/PART
5	4	GC08
6	4	GC05
9	4	GC04

ANGLE TABLE FRAME LINE A	
◇ID	PART
1	BA-24
2	GW-8



BACK SIDEWALL FRAMING: FRAME LINE A

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ANSI D (22"x34")

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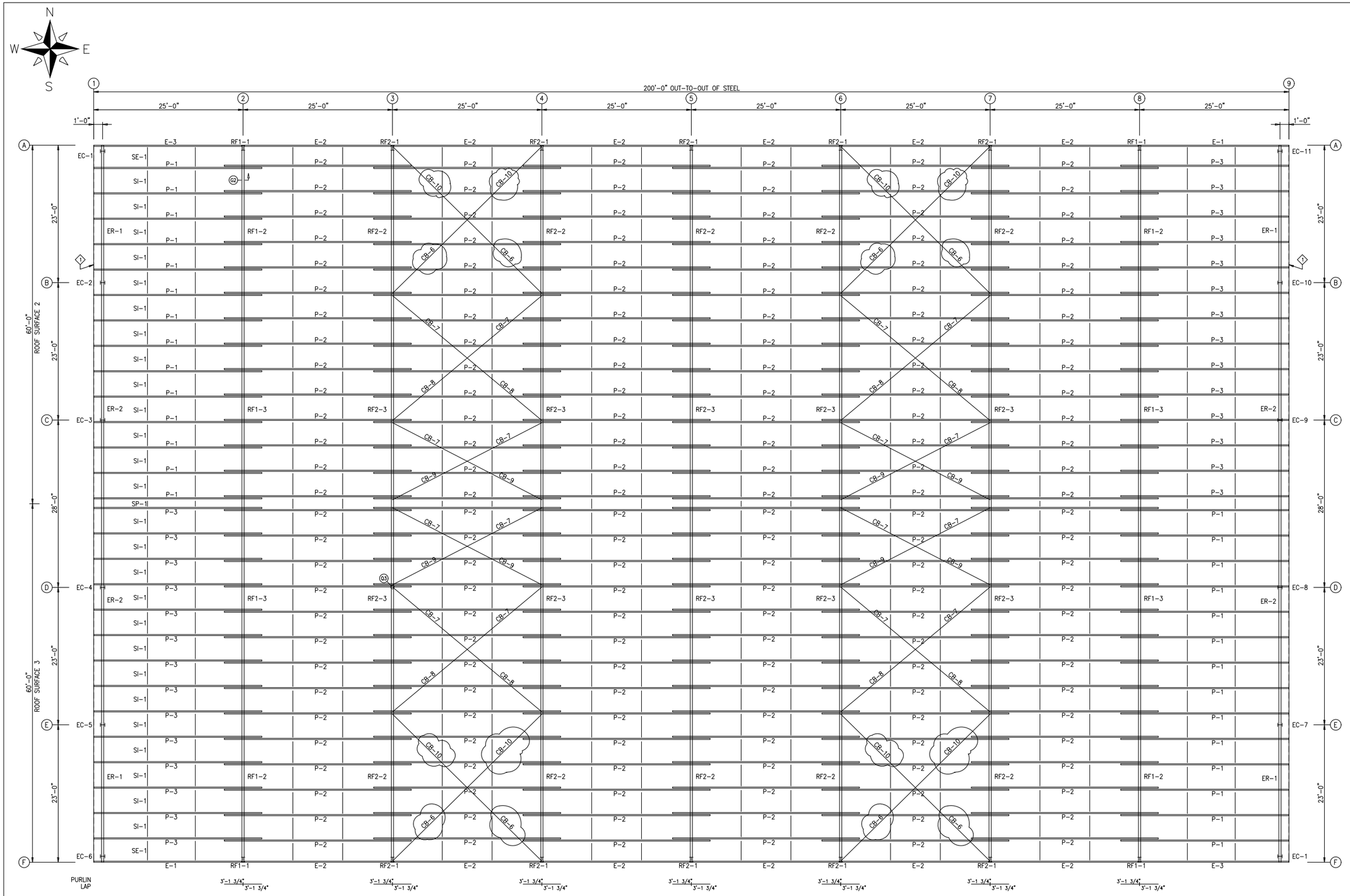
	3378-23T	S10 OF S13		2



MEMBER TABLE ROOF PLAN		
QUAN	MARK	PART
28	P-1	10X25Z12
168	P-2	10X25Z14
28	P-3	10X25Z12
2	E-1	E10X20L2
12	E-2	E10X20L2
2	E-3	E10X20L2
8	CB-10	0.625_ROD
8	CB-6	0.625_ROD
16	CB-7	0.50_ROD
8	CB-8	0.50_ROD
8	CB-9	0.50_ROD
16	SP-1	L100X14
416	SI-1	L100X14
32	SE-1	L100X14

ANGLE TABLE ROOF PLAN	
◇ID	PART
1	RA-24

GENERAL NOTES:

(2) 1/2" Flat Washers at All Purlin Bolts
(6) 1/2" A307 Bolts at 8", 10" & 12" Lapped Purlins
(8) 1/2" A307 Bolts at 14" Lapped Purlins
(2) 1/2" A307 Bolts at EndWall Rafters
(4) 5/8" A325 Bolts at Eavestruts to Rafter
(1) 5/8" A325 Bolt at each end of FB's

NOTE: ENSURE WASHER IS INSTALLED BETWEEN NUT OF STRUT BOLT AND COLD-FORMED EAVESTRUT.

SPECIAL BOLTS (ERECTOR NOTE):

At all "SPECIAL BOLT" locations (as denoted on this dwg. by the symbol (ⓧ), refer to the "SPECIAL BOLT" table on this dwg. for specified bolt grade, quantities, diameter and length.

NOTE: DO NOT SCALE DRAWINGS.

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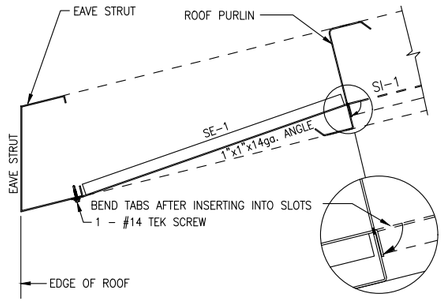
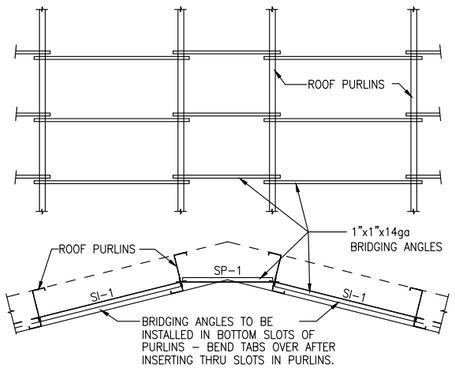
	3378-23T	S11 OF S13		2



PURLIN BRIDGING @ LOW EAVE STRUT

For Screw–Down Roofs (when specified)

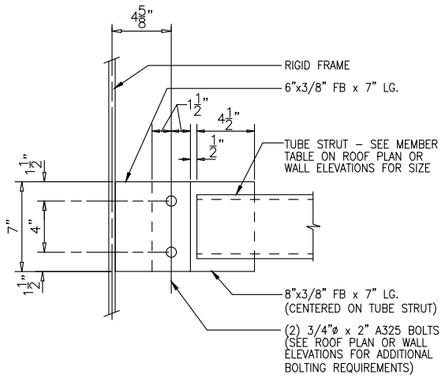
[DETAIL B–Z2]



PURLIN BRIDGING ANGLES AT ROOF PEAK

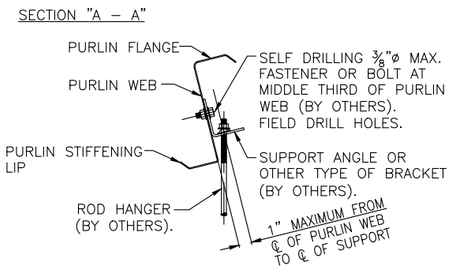
For Screw–Down Roofs (when specified)

[DETAIL B–Z1]



ROOF OR WALL TUBE STRUT TO RIGID FRAME

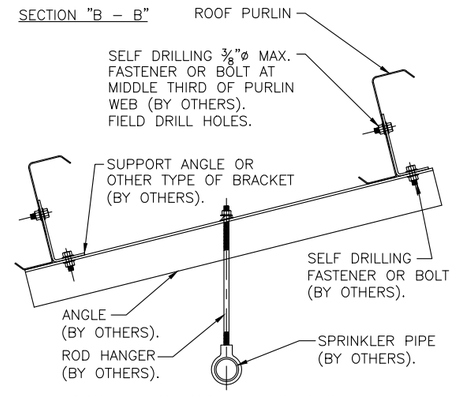
[DETAIL B–Z6]



GENERAL RESTRICTION:
UNDER NO CIRCUMSTANCES CAN THE PURLIN STIFFENING LIP BE FIELD MODIFIED FROM THE FACTORY SUPPLIED CONDITION. DO NOT HANG ANYTHING FROM PURLIN STIFFENING LIP.

SPRINKLER PIPE HANGER DETAIL AT SINGLE PURLIN

[DETAIL B–Z7]



SPRINKLER PIPE HANGER DETAIL BETWEEN PURLINS

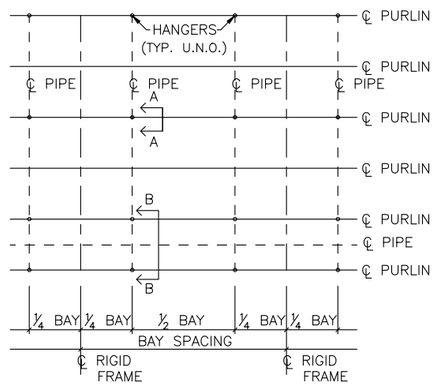
[DETAIL B–Z8]

STD PIPE SIZE	DRY WT. lb/ft	WET WT. lb/ft	EQUIVALENT COLLATERAL LOAD FOR PIPE SUPPORTS, lb/ft²			POINT LOAD AT HANGER lb
			20' BAYS	25' BAYS	30' BAYS	
6 STD	19.0	31.5	8	6	5	315
5 STD	14.6	23.3	5	4	3	233
4 STD	10.8	16.3	4	3	2	163
3 ½ STD	9.12	13.4	4	2	2	134
3 STD	7.58	10.8	3	2	2	108
2 ½ STD	5.80	7.9	2	1	1	79
2 STD	3.66	5.1	1	1	1	51
1 ½ STD	2.72	3.6	1	1	1	36
1 ¼ STD	2.27	2.9	1	1	1	29
1 STD	1.68	2.1	1	1	1	21

NOTES:
1. PIPE HANGERS ASSUMED AT 10' SPACINGS, LOCATED PER TYPICAL PIPE LAYOUT AS SHOWN ON DETAIL B–Z10.
2. FOR PIPES SUPPORTED AT 5' INTERVALS VALUES ABOVE MAY BE HALVED.
3. PURLIN SPACING ASSUMED TO BE 5' ON CENTER.
4. SPECIAL CONSIDERATION SHOULD BE MADE WHEN ALL BAYS ARE NOT UNIFORMLY LOADED.

COLLATERAL LOADS FOR SPRINKLER PIPE SUPPORTS

[DETAIL B–Z9]



TYPICAL SPRINKLER PIPE LAYOUT

[DETAIL B–Z10]

RESERVED FOR ENGINEERING SEALS

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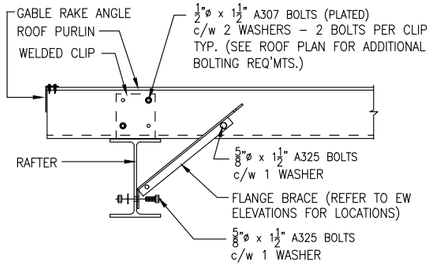
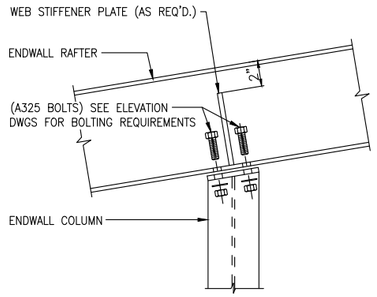
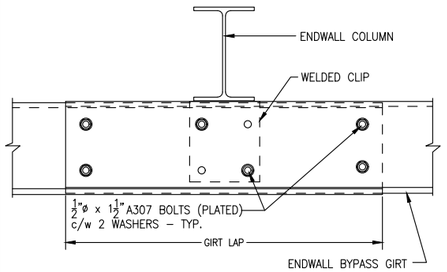
	3378–23T	S12 OF S13		2



A7 ROOF PURLIN TO ENDWALL RAFTER

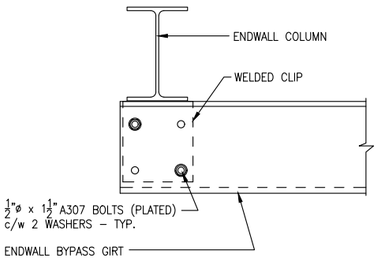


B3 ENDWALL RAFTER TO COLUMN



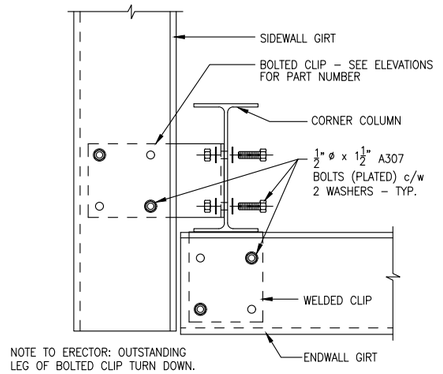
NOTE: FLANGE BRACES (NOT SHOWN) MAY BE PRESENT

C12 BYPASS WALL GIRT TO ENDWALL COLUMN



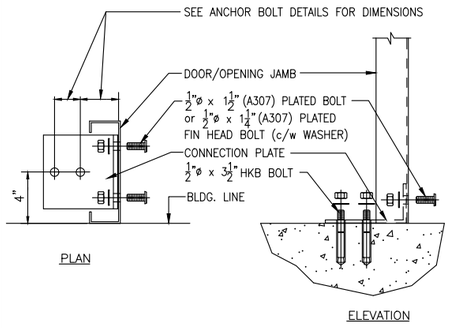
NOTE: FLANGE BRACES (NOT SHOWN) MAY BE PRESENT

C72 BYPASS WALL GIRT TO ENDWALL COLUMN

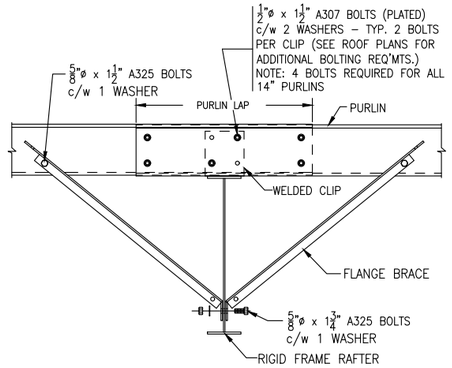


NOTE TO ERECTOR: OUTSTANDING LEG OF BOLTED CLIP TURN DOWN.

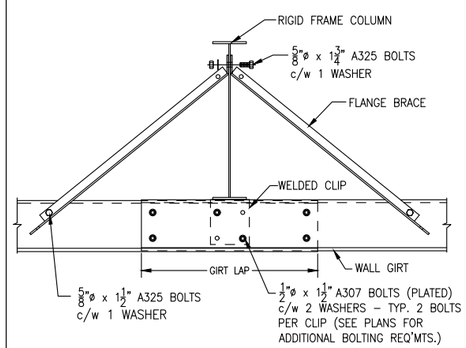
D12 CORNER COLUMN TO WALL GIRT



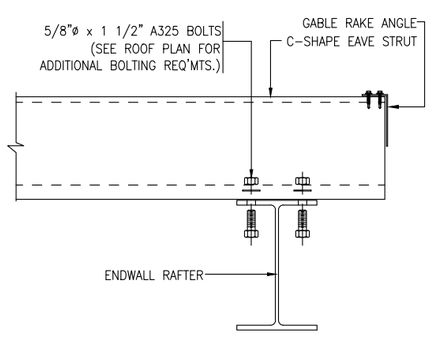
E6 BASE PLATE FOR DOOR JAMB



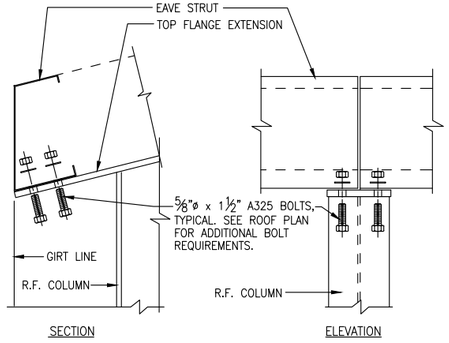
G2 ROOF PURLIN TO INTERIOR R.F. RAFTER



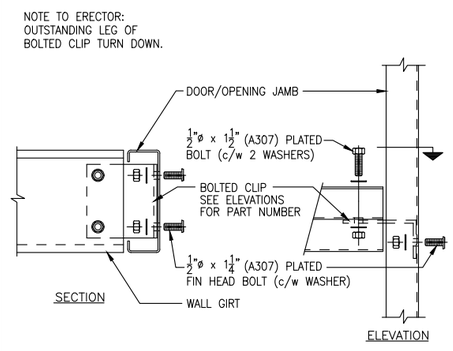
H2 WALL GIRT TO R.F. COLUMN



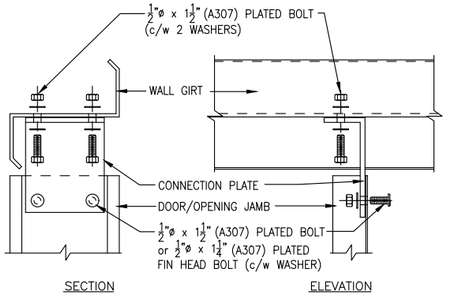
I8 C-SHAPE EAVE STRUT TO ENDWALL RAFTER (@ LOW EAVE)



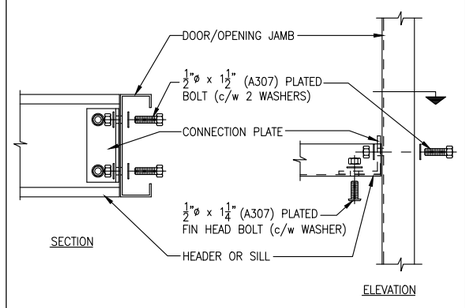
J2 EAVE STRUT TO INTERIOR RIGID FRAME (@ LOW EAVE)



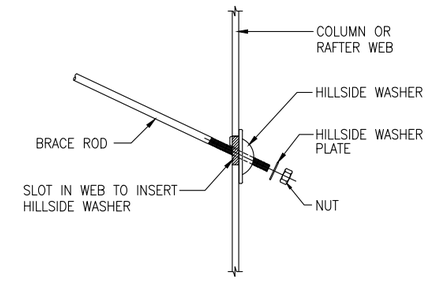
K2 WALL GIRT TO DOOR JAMB



L6 DOOR JAMB TO WALL GIRT



M1 DOOR HEADER TO DOOR JAMB



Q3 DIAGONAL BRACE ROD (NUT END)

PERMIT TO PRACTICE
NOTCH1 MANAGEMENT LTD.
Signature
Date 3 September 2024
PERMIT NUMBER: 14770
The Association of Professional Engineers, Geologists and Geophysicists of Alberta

PROFESSIONAL ENGINEER ALBERTA KEVIN LACROIX

RESERVED FOR ENGINEERING SEALS

No.	REVISION	DATE	BY	CHKD
0	ISSUED FOR APPROVAL	3/29/23	SG	CP
1	ISSUED FOR FABRICATION	4/11/23	SG	CS
2	ISSUED FOR FABRICATION	3/09/24	MBB	CP

No.	REVISION	DATE	BY	CHKD

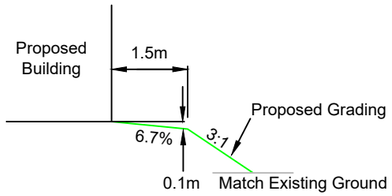
BRYTEX™
BRYTEX BUILDING SYSTEMS INC.

CUSTOMER: NORTHERN STEEL BUILDINGS	DESIGN BY:	DRAWN BY:	CHKD BY:	DATE:
PROJECT: HIGH LEVEL AG SOCIETY	KL	SG	CP	3/28/23
LOCATION: HIGH LEVEL, ALBERTA	JOB NO.:	SHEET No.:		REV.
	3378-23T	S13 OF S13		2

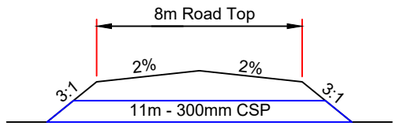
SCHEDULE B

Approved September 10, 2025

Viv Thoss
Development Authority



DETAIL "A" (not to scale)
Proposed Grading Along Building

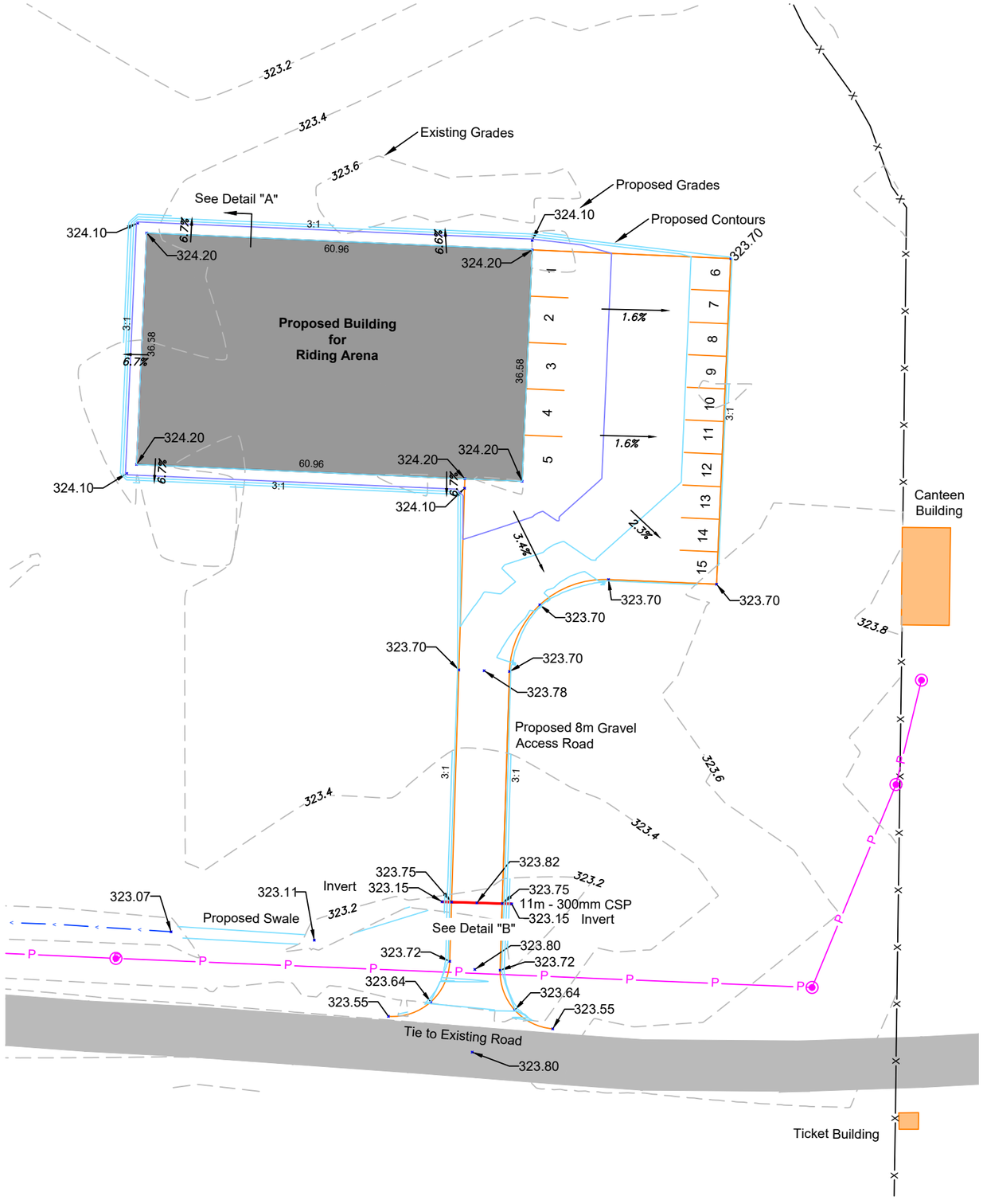


DETAIL "B" (not to scale)
Road and Culvert Crossing

Rough Volume Calc.

Assumptions:
200mm of Topsoil Depth
150mm Gravel on Parking and Road
150mm Prep in Building.

Area of Construction = 5270m^2
Topsoil Removal = 5270*0.20 = 1054m^3
Fill Required = 3040m^3
Gravel Required = 415m^3
Building Prep. = 375m^3



SITE PLAN SHOWING PROPOSED GRADING PLAN FOR

EQUESTRIAN RIDING ARENA

Within

S.W. $\frac{1}{4}$ Sec.3, Twp.110, Rge.19, W.5M.

Within

Town of High Level, Alberta

N

0 10 20 40 60 80

SCALE 1:750

Notes

C:\Users\trevo\Borderline Surveys Dropbox\DRAFTING\LOGO\borderline logo.jpg

10202 99th Street
La Crete, Alberta, T0H 2H0
Phone: (780) 538-1955
E-mail: jwc.surveyor@gmail.com

ALBERTA LAND SURVEYORS' ASSOCIATION
PERMIT NUMBER
P298
BORDERLINE SURVEYS LTD.

PREPARED BY
Jason Coates, A.L.S.

Revision Table

No.	Revision Type	Drafted	Chk'd	Surveyed	Date
0	Original	MM	LB/JC	JC	July 2, 2025

Client File No: N/A

File No: 250114 | Job No: 250114 | Sheet: 3 of 3

1 Revision